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# Evaluating the Performance and Effectiveness of Roundabouts

Presented by:

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# Study Objectives

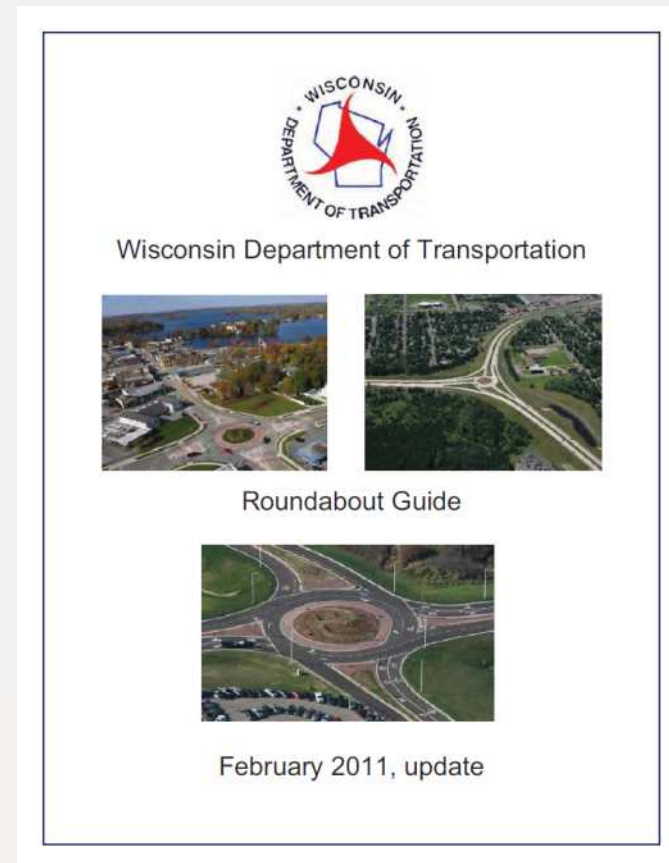
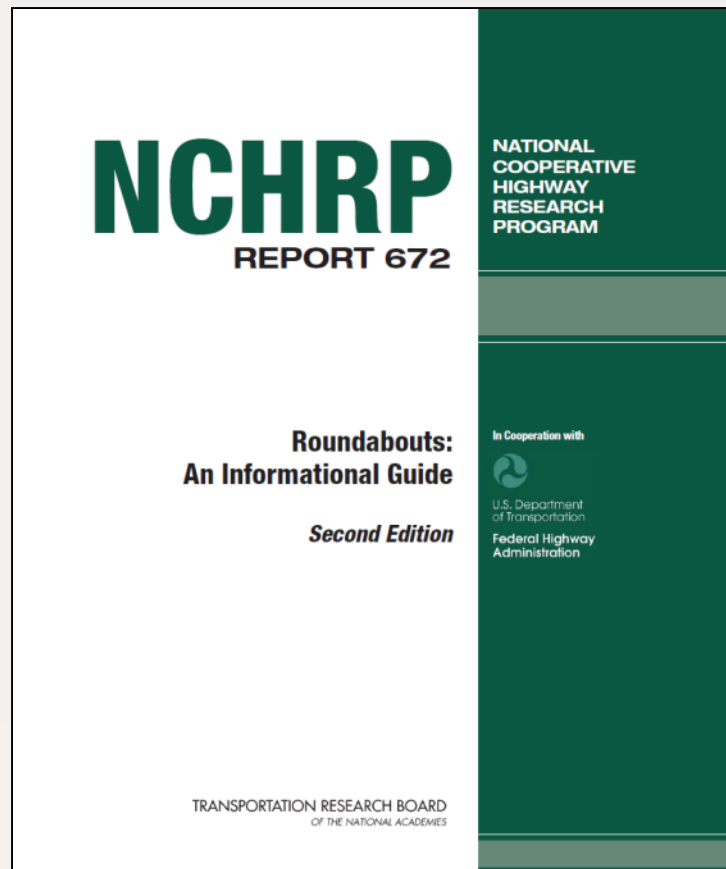
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- Determine the impact on crashes at locations where roundabouts have been installed
- Observe roundabout operations including truck maneuvers
- Identify the key geometric configurations and site characteristics that influence safety, performance and return on investment



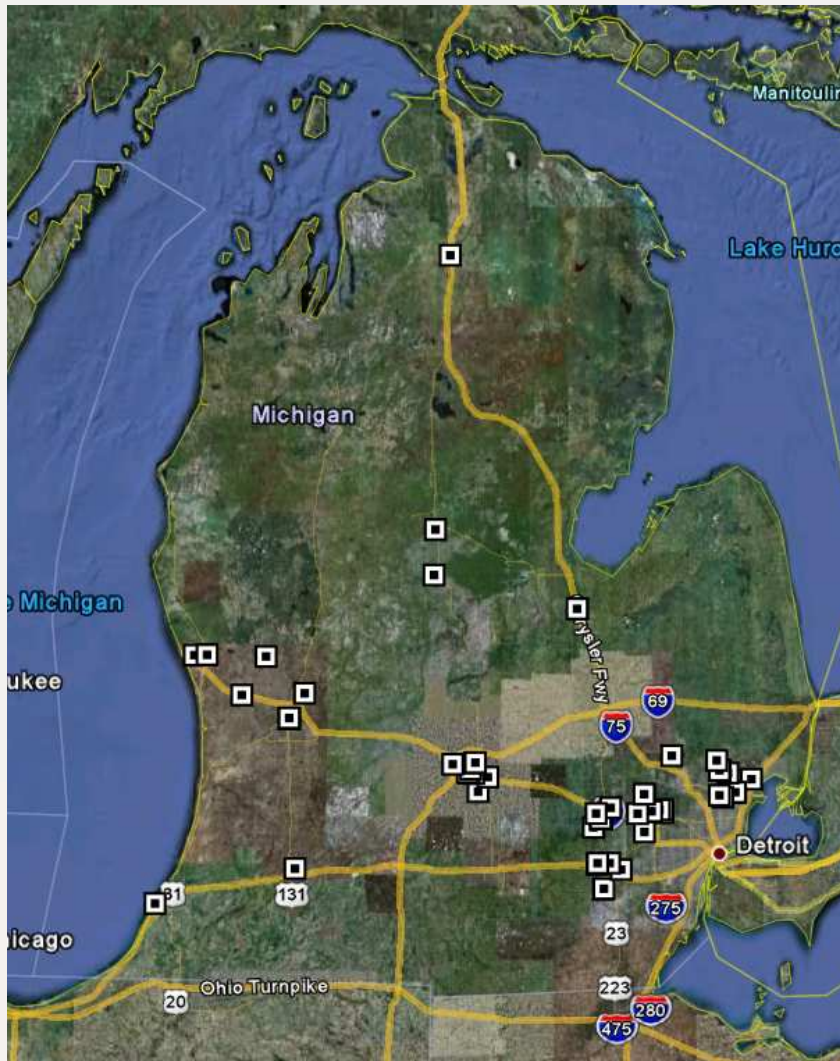
# Literature & Best Practices Review

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# Identify Roundabouts

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**97 Roundabouts  
identified in Michigan**

**58 Roundabouts included in  
the analysis**

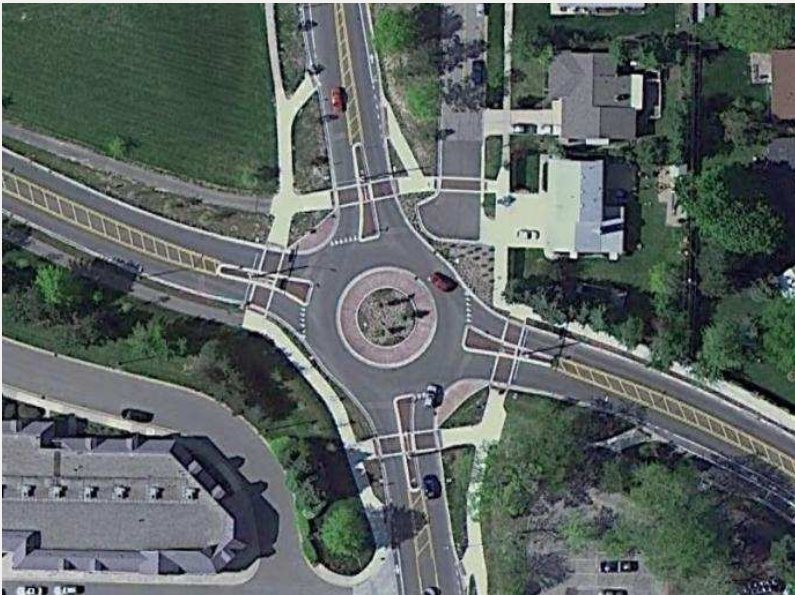
- 47 Converted Sites
- 11 New Build Sites

**15 sites - detailed reviews**



# Site Visits

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- Walk-through
- Review of geometric design
- Observe operations
- Consider road user visibility at night
- Review truck turning maneuvers
- Review traffic control devices
- Treatment and transition of non-motorized facilities
- Modifications made by MDOT to roundabouts
- Traffic Counts
- Speed Studies





# Traffic Conflicts

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- “Near Misses”
- Key surrogate measure for safety performance
- Collected at following roundabouts;
  - M-47 & M-37
  - NB M-53 Ramp & 26 Mile
  - SB M-53 Ramp & 26 Mile
  - M-53/Van Dyke & 18 1/2 Mile



# Data Collection

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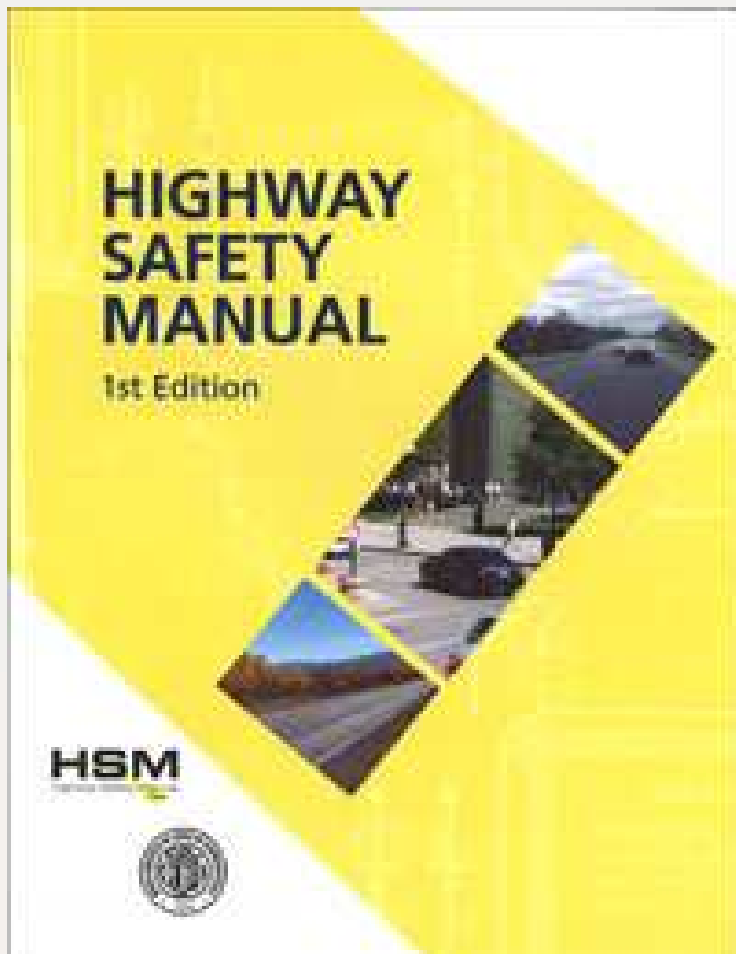
- Crash data at treatment sites – before/after
- Crash data at reference sites – before/after
- Traffic Volumes - before/after
- Geometric and operational characteristics
- Implementation Dates



# Safety Analysis

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## *Develop Safety Performance Functions*



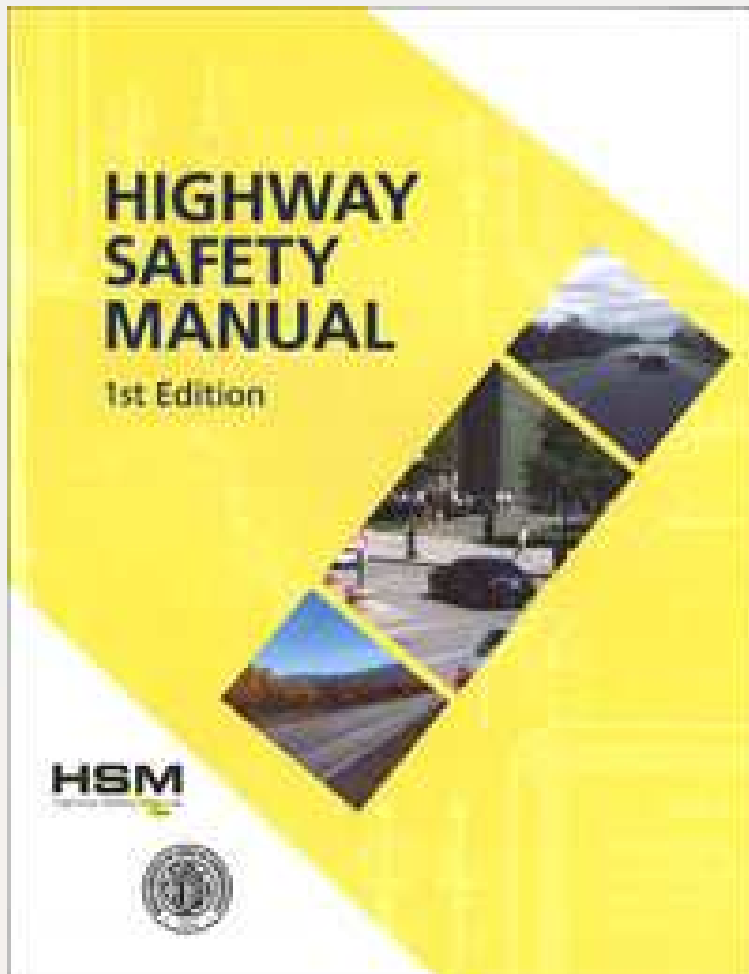
- Utilized sites without roundabouts (stop/signal controlled, urban/rural) for the Empirical Bayes (EB) analysis
- Attempted to calibrate new SPFs with Michigan data, but was unsuccessful at a 90% confidence interval or greater
- Utilized the HSM calibration procedure to adapt *SafetyAnalyst* SPFs



# Safety Analysis

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## *Empirical-Bayes Analysis*



- Estimates change in crashes by severity (total and fatal+injury crashes)
- Uses SPFs to account for regression-to-mean, crash trends, changes in traffic
- Scientifically rigorous – *SafetyAnalyst* and HSM Methodology



# Safety Analysis

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*Two-Way Stop → Roundabout*



*US-127BR & Mission Street  
Clare County*

- Total Crashes=+9.5%
- Fatal & Injury Crashes=-42%



# Safety Analysis

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*Two-Way Stop at Interchange → Roundabout*



*M-14 & Maple Road Interchange  
Washtenaw County*

- Total Crashes=+24%
- Fatal & Injury Crashes=-58%



# Safety Analysis

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*Signal → Signal or Double Lane Roundabout*



*Utica & Dodge Park  
Macomb County*

- Total Crashes=-22%
- Fatal & Injury Crashes=-70%





# Safety Analysis

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*Signal → Triple Lane Roundabout*



*14 Mile & Farmington  
Oakland County*

- Total Crashes+98%
- Fatal & Injury Crashes=-20%





# Safety Analysis

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Site Type by Condition before conversion	CMF	
	Total	Injury
All sites	1.346	0.583
All sites minus triple lane	1.002	0.488
One or two way Stop-controlled (All)	1.117	0.558
One or two way stop controlled at interchange	1.247	0.419
One or two way stop controlled not at interchange	1.095	0.581
All-way stop-controlled	1.026	0.636
Signalized (single and double lane roundabouts)	0.783	0.300
Signalized (triple lane roundabouts)	1.975	0.801

# Safety Analysis

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$$\text{Crashes/year} = \exp^{\alpha} (\text{AADT})^{\beta_1} \exp^{(\beta_2 * \text{Type} + \text{IC} * \beta_3)}$$

Model	$\alpha$ (s.e.)	$\beta_1$ (s.e.)	$\beta_2$ (s.e.)	$\beta_3$ (s.e.)	k (s.e.)
Total	1 lane -4.5958 (1.2851) 2 lane -3.8074 (1.2621)	0.5253 (0.1274)	-0.7884 (0.2423)	0.6988 (0.3710)	0.4839 (0.1266)
Injury	1 lane: -6.4109 (1.8322)  2 lane: -5.7287 (1.8066)	0.4788 (0.1795)	-0.6822 (0.3051)	0.7850 (0.5733)	0.2460 (0.1933)

# Operational Analysis

# Calculate change in LOS and delay at select treatment sites using RODEL software

```

C:\WINDOWS\system32\cmd.exe
9:11:07 HWY 1 WB OFF RAMP/MCCALLUM OUT FINAL 17
E (m) 7.70 4.25 7.70 7.70 TIME PERIOD min 90
L' (m) 25.00 10.00 25.00 25.00 TIME SLICE min 15
U (m) 7.30 3.65 7.30 7.30 RESULTS PERIOD min 15 75
RAD (m) 20.00 20.00 20.00 20.00 TIME COST $/hr 15.00
PHI (d) 30.00 30.00 30.00 30.00 FLOW PERIOD min 15 75
DIA (m) 40.00 40.00 40.00 40.00 FLOW TYPE pcu/veh UEH
GRAD SEP 0 0 0 0 FLOW PEAK am/op/pm PM

LEG NAME PCU UEH TURNS (1st exit, 2nd..U) FLOF CL FLOW RATIO FLOW TIME
SOUTHBOUND 1.05 000 933 000 0 1.00 50 0.75 1.125 0.75 15 45 75
EASTBOUND 1.05 000 000 000 0 1.00 50 0.75 1.125 0.75 15 45 75
NORTHBOUND 1.05 000 715 000 0 1.00 50 0.75 1.125 0.75 15 45 75
WESTBOUND 1.05 434 000 129 0 1.00 50 0.75 1.125 0.75 15 45 75

MODE 2
FLOW veh 933 0 715 563 AUEDEL s 2.8
CAPACITY veh 2117 611 2216 1668 LOS SIG A
AVE DELAY mins 0.05 0.00 0.04 0.05 LOS UNSIG A
MAX DELAY mins 0.07 0.00 0.05 0.07
AVE QUEUE veh 1 0 0 1 UEHIC HRS 1.7
MAX QUEUE veh 1 0 1 1 COST $ 26

F1mode F2direct F3peak CtrlF3rev F4fact F6stats F8econ F9prnt F10run Esc

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# Operational Analysis

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AM Peak Period					
Intersection	“Before”		“After”		% change
	LOS	Delay (sec/veh)	LOS	Delay (sec/veh)	
EB M-14 Ramps at Maple Road	F	105.5	A	6.5	93.8%
WB M-14 Ramps at Maple Road	D	34.2	A	5.1	85.1%
Huron Parkway at Nixon Road	B	22.3	A	5.7	74.4%
Maple Road at Drake Road	D	35.0**	A	5.7	83.7%
Maple Road at Farmington Road	F	80.0**	A	7.7	90.4%
** denotes an assumed value					



# Operational Analysis

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PM Peak Period					
Intersection	"Before"		"After"		% Change
	LOS	Delay (sec/veh)	LOS	Delay (sec/veh)	
EB M-14 Ramps at Maple Road	E	37.1	A	6.5	82.5%
WB M-14 Ramps at Maple Road	C	16.0	A	3.7	76.9%
Huron Parkway at Nixon Road	B	19.5	A	6.8	65.1%
Maple Road at Drake Road	E	55.0**	A	6.4	88.4%
Maple Road at Farmington Road	E	55.0**	A	9.5	82.7%
** denotes an assumed value					





# Economic Analysis

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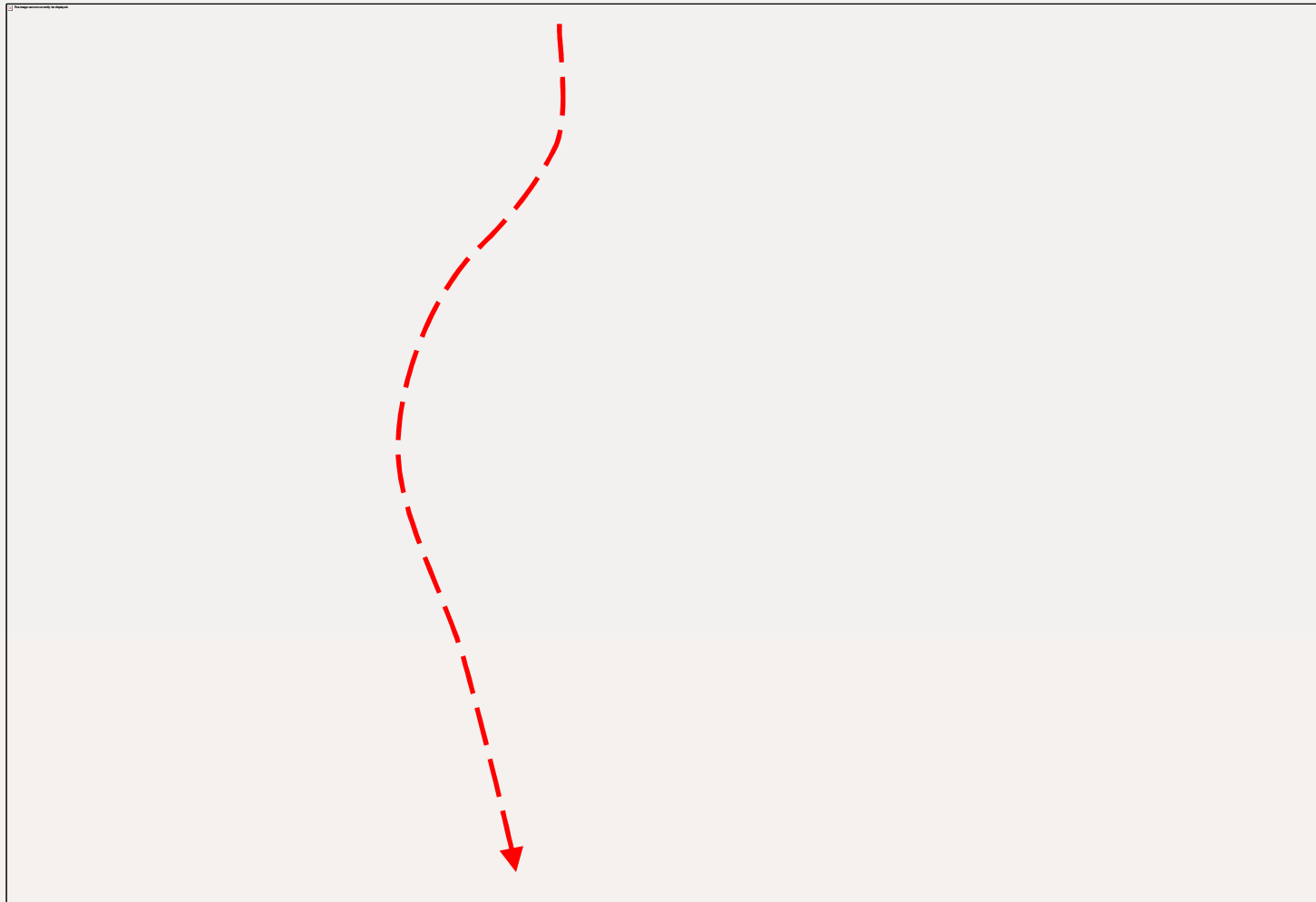
Type	Time of Return (years)
Single Lane Roundabout	0.8
Double Lane Roundabout	1.2
Triple Lane Roundabout	0.9



# Findings

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*Lane Discipline → Higher Speeds*



# Findings

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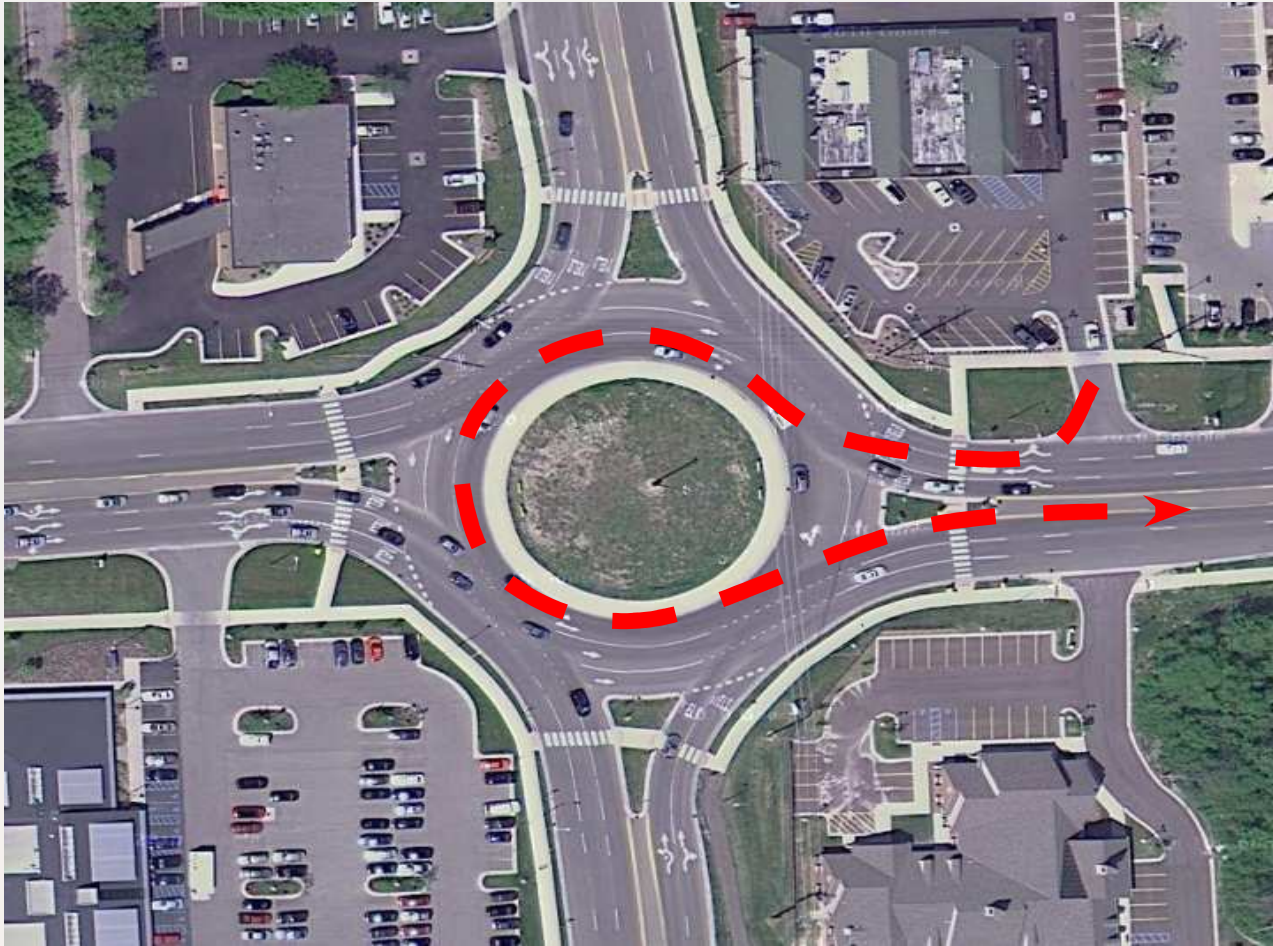
## *Merging on Exit Legs*



# Findings

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## *Driveways Near Roundabouts*



# Contact

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